

GMS, INC.
CONSULTING ENGINEERS
611 NORTH WEBER, SUITE 300
COLORADO SPRINGS, COLORADO 80903-1074

TELEPHONE (719) 475-2935
TELEFAX (719) 475-2938

EDWARD D. MEYER, P.E.
ROGER J. SAMS, P.E.
GREGORY R. WORDEN, P.E.
THOMAS A. McCLENNAN, P.E.

KEN L. WHITE, P.L.S.
DAVID R. FRISCH, P.L.S.
MARK A. MORTON, P.E.
JASON D. MEYER, P.E.

April 10, 2013

Mr. Wes Weaver, President
Weaver Construction Management, Inc.
c/o Garney Construction
7911 Shaffer Parkway
Littleton, CO 80127

*Via Email to: wes@weavercm.com
No Hard Copy to Follow*

Re: Harold D. Thompson Regional Water Reclamation Facility (HDTRWRF)
Lower Fountain Metropolitan Sewage Disposal District (LFMSDD)

Dear Wes:

Reference is made to your shop submittal identified as follows:

Submittal No.:	02500-002
Date of Submittal:	April 10, 2013
Title:	Asphalt Paving, Grading S and SX
Specification Section:	02500
Manufacturer:	Schmidt Construction Company

The referenced submittal has been stamped "**No Exception Taken**". No additional information or resubmittal is required on this item.

Please call if you should have any questions.

Sincerely,



David R. Frisch, P.L.S.

DRF/kmw

ec (letter only):

Mr. Jim Heckman, Manager, LFMSDD, lfmanager@lfmsdd.org
Ms. Cindy Murray, Office Manager, Fountain Sanitation District, fsdistrict@fsd901.org
Mr. Jeff Burst, Project Supt., Weaver Construction Management, Inc., jeff@weavercm.com
Mr. John Jacob, Project Mgr., Weaver Construction Management, Inc., john@weavercm.com
Mr. Adam Roeder, Weaver Construction Management, Inc., aroeder@weavercm.com
Ms. Solange Huggins, Project Engineer, Garney Construction, shuggins@garney.com
cc (w/attachments): Mr. Jerry Miller, Resident Project Representative, GMS, Inc.



Weaver

CONSTRUCTION MANAGEMENT

7911 Shaffer Parkway Littleton, CO 80127
(P) 303-791-3600 (F) 303.791.1801

SUBMITTAL TRANSMITTAL

April 10, 2013

Submittal No: 02500-02

PROJECT: **Harold Thompson Regional WRF**
Birdsall Rd.
Fountain, CO 80817
Job No. 2908

ENGINEER: **GMS, Inc.**
611 No. Weber St., #300
Colorado Springs, CO 80903
719-475-2935 Roger Sams

OWNER: **Lower Fountain Metropolitan
Sewage Disposal District**
901 S. Santa Fe Ave.
Fountain, CO 80817
719-382-5303 James Heckman

CONTRACTOR: Schmidt Construction Company
2635 Delta Drive
Colorado Springs, Colorado 80910
Phone 719-395-4207

SUBJECT: Asphalt Paving 02500 Both Grading S and SX - Schmidt

SPEC SECTION: 02500 Paving and Surfacing

PREVIOUS SUBMISSION DATES:
DEVIATIONS FROM SPEC: ___ YES X NO

CONTRACTOR'S STAMP: This submittal has been reviewed by Weaver Construction Management and, unless indicated otherwise, has been found to be in conformance with the intent of the contract documents.

Contractor's Stamp:

Engineer's Stamp:

Date: 4/10/13

Reviewed by: John Jacob

(X) Reviewed Without Comments
() Reviewed With Comments

ENGINEER'S
COMMENTS: _____

2013 C/S MIX SPECS

SUNCOR PG 64-22		DELTA YARD MIXES									
2013		SR		SR PPRAS	SXR		SXR PPRAS	SXFR	SXFR PPRAS		
PRODUCT CODE		2560	2559	2559	2563	2562	2562				
		S100R6422-	S75R6422-	S75R6422-	SX100R6422-	SX75R6422-	SX75R6422-	SXF75R642	SXF75R642		
MIX #		13	13	13	13	13	13	2-13	2-13		
GYRATION LEVEL		100	75	75	100	75	75	75	75		
BINDER		64-22	64-22	64-22	64-22	64-22	64-22	64-22	64-22		
AC OPTIMUM		5.1	5.3	5.5	5.5	5.8	6.0	5.7	5.9		
CDOT AC SPEC		4.8-5.4	5.0-5.6		5.2-5.8	5.5-6.1		5.4-6.0			
PPRAS AC SPEC		4.7-5.5		5.1-5.9	5.1-5.9		5.6-6.4		5.5-6.3		
IGNITION CORRECTION		-0.08			-0.10			-0.04			
CDOT/PPRAS JOB		MIX FORMULA TOLERANCES									
								- SUBTRACT FROM			
								+ ADD TO			
GRADATION	1"	100		Virgin	100		Virgin	100		1"	GRADATION
+/-6	3/4"	100		100	100		100	100		100	3/4"
+/-6	1/2"	83-95		87	90-100		95	100		100	1/2"
+/-6	3/8"	73-85		76	83-95		89	89-100		96	3/8"
+/-5	#4	58-68		60	61-71		65	63-73		67	#4
+/-5	#8	42-52		44	44-54		47	46-56		48	#8
	#16			31			33			35	#16
+/-4	#30	20-28		22	22-30		24	22-30		25	#30
+/-4	#50	13-21		15	13-21		16	14-22		17	#50
	#100			10			10			11	#100
+/-2	#200	4.7-8.7		6.3	5.0-9.0		6.7	5.1-9.1		6.9	#200
RICE		2.461	2.453	2.446	2.445	2.435	2.426	2.438	2.431	RICE	
LBS/FT ³		153.6	153.1	152.6	152.6	151.9	151.4	152.1	151.7	LBS/FT ³	
AGG BULK SP.G.		2.618			2.618					AGG BULK SP.G.	
LABORATORY MIXING TEMP/		325F			325F			325F			
COMPACTION TEMP		300F			300F			300F			
AGGREGATE BLENDS											
FOUNTAIN 1/2" HMA		20			20			20			
MENZER 3/4" ROCK		22			8						
MENZER 1/2" ROCK					10			16			
MENZER GRANITE SAND		37			41			43			
RAP		20			20			20			
LIME		1			1			1			



2/18/2013
Schmidt Quality Control Laboratory
Schmidt Construction Company
Colorado Springs, Colorado 80910
AMRL/AASHTO Accredited Laboratory No. 2531

Subject: Hot Mix Asphalt Mix Design
SHRP Design Method – 75 Gyration
Grading S w/RAP PG64-22
Delta Plant
Mix Design No. S75R6422-13

Attached are the results of a hot mix asphalt mix design performed in accordance with AASHTO, CDOT and/or ASTM procedures, utilizing the SHRP method for specimen compaction. Aggregates used in the mix design are from Menzer and Fountain Quarries. The asphalt binder is a PG 64-22, with a specific gravity of 1.038, a mixing temperature of 325F, and a compaction temperature of 300F, supplied by SUNCOR Materials. The anti-stripping additive used is 1% hydrated lime from Pete Lien.

The aggregate was blended to meet CDOT Grading S criteria by blending 20% Fountain 1/2" HMA, 37% Menzer Asphalt Fines, 22% Menzer 3/4" Rock, 20% Crushed RAP and 1% Hydrated Lime. The individual gradations, combined blend and aggregate physical properties are presented on page one (1) and a graphical presentation of the volumetric properties and the combined gradation, plotted on a 0.45 Power Graph, begins on page four (4).

This design was performed using SHRP Mix Design guidelines. The design was performed at asphalt contents of 4.5, 5.0, 5.5 and 6.0 percent. The results of the tests performed at each asphalt content are presented on pages two (2) and three (3) of the design. Optimum values for our Standard Mix Design are chosen at 4.0% VTM and are presented on page two (2). Optimum values for the PPRAS Criteria Mix Design are chosen at 3.5% VTM and are presented on page three (3). Based on these results the optimum properties of this mix are:

	VALUES @	STANDARD OPTIMUM	PPRAS OPTIMUM
Optimum AC Content, %		5.3	5.5
Theoretical Maximum Specific Gravity		2.453	2.446
Air Voids, %		4.0	3.5
Voids in Mineral Aggregate, %		14.8	14.8
Voids Filled with Asphalt, %		73	77
Tensile Strength Ratio		94	94

This hot mix asphalt design is based on specific materials and laboratory preparation of the test specimens. Variation between laboratory-produced and field-produced samples should be anticipated. It is recommended that the mix design be field verified during initial production. Field verification often results in the optimum asphalt cement content being adjusted to meet design air voids or voids in the mineral aggregate criteria.

If you have any questions concerning this mix design, please contact us at (719) 392-4207.

Performed By: Schmidt Construction Testing Laboratory

Reviewed By: Rueben Roberts Rueben Roberts, QC-Mgr

Reviewed By: Tom Blair Tom Blair, E.I.T.

Approved By: Rudy Keng Rudy Keng, P.E.

3/12/2013



SCHMIDT CONSTRUCTION, COLORADO SPRINGS, CO

AMRL/AASHTO Lab No. 2531

Laboratory Design for Hot Bituminous Pavement-SHRP Method

Mix Identification:

Date: 2/18/13

S75R6422-13

Project: 2013 MIX DESIGN S75R

Location:

DELTA PLANT

Contractor/Supplier: Schmidt Construction

Grading: S w/RAP 75 Gyration

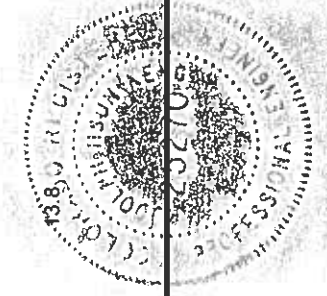
Pit Name: Menzer Quarry / Fountain Pit

AC Source & Grade: SUNCOR PG 64-22

Mixing/Compacting Temperatures: 325F / 300F

Aggregate Data (T-11, T-27, T-30, T-37):

Aggregate Source	Fountain 1/2" HMA Rock	Menzer Asphalt Fines	Menzer 3/4" Rock	Lime	Virgin Aggregate Gradation	RAP	Combined Gradation	Control Points
Percent in Mix	20	37	22	1	80	20	100	
Passing 1 1/2	100 (37.5)	100	100	100	100	100	100	100
Passing 1	100 (25.0)	100	100	100	100	100	100	100
Passing 3/4	100 (19.0)	100	100	100	100	100	100	100
Passing 1/2	100 (12.5)	100	53	100	87	99	89	
Passing 3/8	95 (9.5)	100	16	100	76	93	79	
Passing 4	75 (4.75)	86	2	100	60	73	63	
Passing 8	56 (2.36)	61	1	100	44	60	47	23
Passing 16	39 (1.18)	43	1	100	31	45	34	
Passing 30	26 (0.60)	31	1	100	22	33	24	
Passing 50	15 (0.30)	21	1	100	15	23	17	
Passing 100	8 (0.15)	13	1	100	10	14	10	
Passing 200	5.5 (0.075)	7.6	0.7	98.6	6.3	8.0	6.7	2.0
Plastic or Non-Plastic (T-90)	N/P	N/P	N/P					
Aggregate Bulk SpG (T-84 & T-85)	2.582	2.621	2.611	2.380			2.618	
Aggregate App. SpG (T-84 & T-85)	2.650	2.680	2.661	2.380			2.665	
Agg Water Abs (%) (T-84 & T-85)	0.98	0.85	0.72				0.82	
Aggregate Eff. SpG (T-84 & T-85)						2.671	2.656	
Fine Agg. Bulk SpG (T-84)							2.607	
Coarse Agg. Bulk SpG (T-85)							2.611	
Binder SpG							1.038	
Fractured Faces (CP-45, ASTM D-5821)							90	60%
Sand Equivalent (T-176)							62	45
LA Abrasion (T-96)	37.6	33.6	30.9				32	45
Fine Aggregate Angularity (T-304)							47	45
Micro-Deval (CP-L 4211)	10.5	16.2					14	18%
Sodium Soundness (T-104)							1.0	12%
Flat & Elongated - Ratio 3:1 (ASTM D-4791)							0.2	
Flat & Elongated - Ratio 5:1 (ASTM D-4791)							0.0	



Standard

SCHMIDT CONSTRUCTION, COLORADO SPRINGS, CO
 Laboratory Design for Hot Bituminous Pavement-SHRP Method
 Standard Mix Design Criteria

Lab No. 2531
 Version

Mix Design A.C. Content Determination Results:

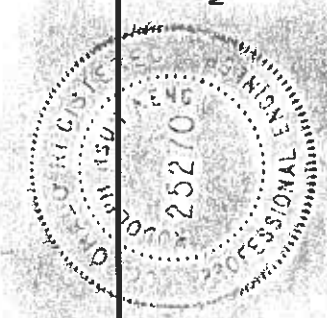
Rice =	2.446	@	5.5	%AC				
A.C. Content			4.5		5.0	5.5	6.0	%
Rice Data (CP-51, T-209)			2.482		2.464	2.446	2.429	
Specimen SpG Data (CP-L5115 & CP-L5106):								
Bulks SpG @ Ninit			2.143		2.146	2.174	2.185	
Bulks SpG @ Ndes			2.302		2.342	2.362	2.363	
Height @ Ndes			65.1		64.1	63.2	62.9	mm
Voids Data:								Voids Specs:
Voids @ Ninit			13.6		12.9	11.1	10.0	%
Voids @ Ndes			7.2		4.9	3.5	2.7	%
Other Data:								Min
VMA @ Ndes (CP-48)			16.0		15.0	14.8	15.1	%
VFA @ Ndes			55		67	77	82	%
Aggregate Effective SpG (T-84 & T-85)			2.656		2.656	2.656	2.656	
Effective Asphalt Content			3.96		4.46	4.96	5.47	%
Dust to A.C. Ratio (CP-50)			1.4		1.3	1.1	1.0	
Stability (CP-L5106)			41		41	40	39	
								Max
								13.7%
								65%
								80%
								0.6
								30

Optimum A.C. Content Results:

Optimum A.C. Content	5.3	%	Voids at Ninit at Optimum A.C.	11.8	%
Rice at Optimum A.C.	2.453		Voids at Ndes at Optimum A.C.	4.0	%
Hveem Stability	40		VMA at Optimum A.C.	14.8	%
Voids Filled with Asphalt (VFA)	73	%	Dust to Asphalt Ratio	1.2	

Lottman Moisture Sensitivity Results (CP-L 5109, Method B):

Asphalt Content (percent)	5.3	%	Lottman Specs:	
Tensile Strength Retained (percent)	94	%	Min	80%
Avg. Dry Tensile Strength (psi)	83.5	psi		30psi
Avg. Cond. Tensile Strength (psi)	78.3	psi		
Avg. Specimen Voids (percent)	6.8	%		
Avg. Saturation (percent)	96.5	%		8.0%



SCHMIDT CONSTRUCTION, COLORADO SPRINGS, CO
 Laboratory Design for Hot Bituminous Pavement-SHRP Method
 Pikes Peak Regional Asphalt Specification Mix Design Criteria

Lab No. 2531

Version

Mix Design A.C. Content Determination Results:

Rice =	2.446	@	5.5	%AC	
A.C. Content	4.5	5.0	5.5	6.0	%
Rice Data (CP-51, T-209)	2.482	2.464	2.446	2.429	

Specimen SpG Data (CP-L5115 & CP-L5106):

Bulks SpG @ Ninit	2.143	2.146	2.174	2.185	
Bulks SpG @ Ndes	2.302	2.342	2.362	2.363	
Height @ Ndes	65.1	64.1	63.2	62.9	mm

Voids Data:

Voids @ Ninit	13.6	12.9	11.1	10.0	%
Voids @ Ndes	7.2	4.9	3.5	2.7	%

Other Data:

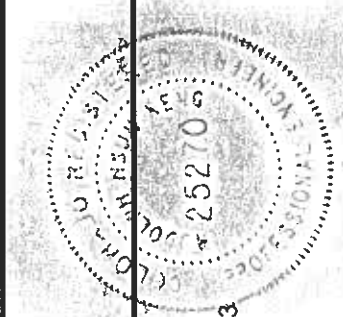
VMA @ Ndes (CP-48)	16.0	15.0	14.8	15.1	%
VFA @ Ndes	55	67	77	82	%
Aggregate Effective SpG (T-84 & T-85)	2.656	2.656	2.656	2.656	
Effective Asphalt Content	3.96	4.46	4.96	5.47	%
Dust to A.C. Ratio (CP-50)	1.4	1.3	1.1	1.0	
Stability (CP-L5106)	41	41	40	39	

Optimum A.C. Content Results: PPRAS Criteria

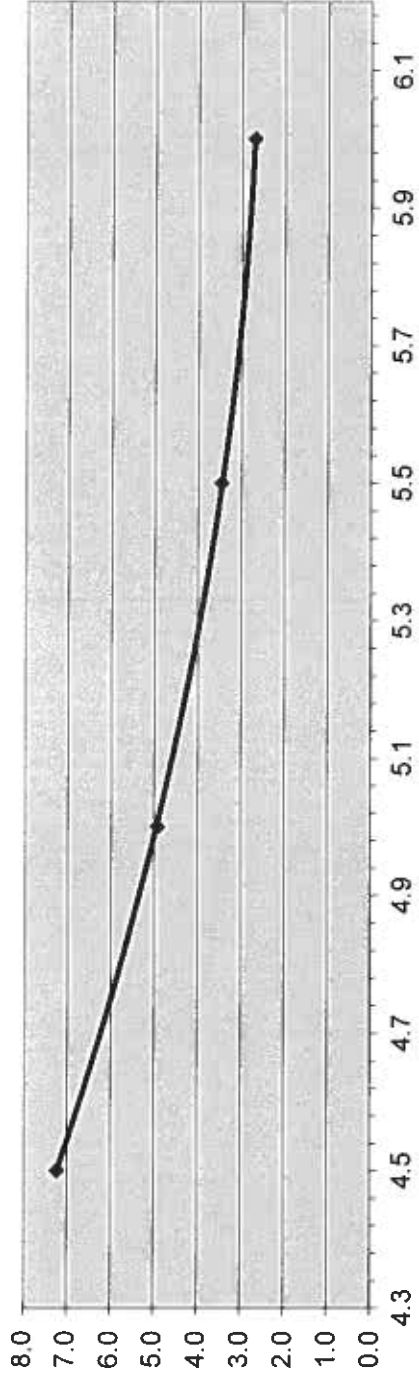
Optimum A.C. Content	5.5	%	Voids at Ninit at Optimum A.C.	11.1	%
Rice at Optimum A.C.	2.446		Voids at Ndes at Optimum A.C.	3.5	%
Hveem Stability	40		VMA at Optimum A.C.	14.8	%
Voids Filled with Asphalt (VFA)	77	%	Dust to Asphalt Ratio	1.1	

Lottman Moisture Sensitivity Results (CP-L 5109, Method B):

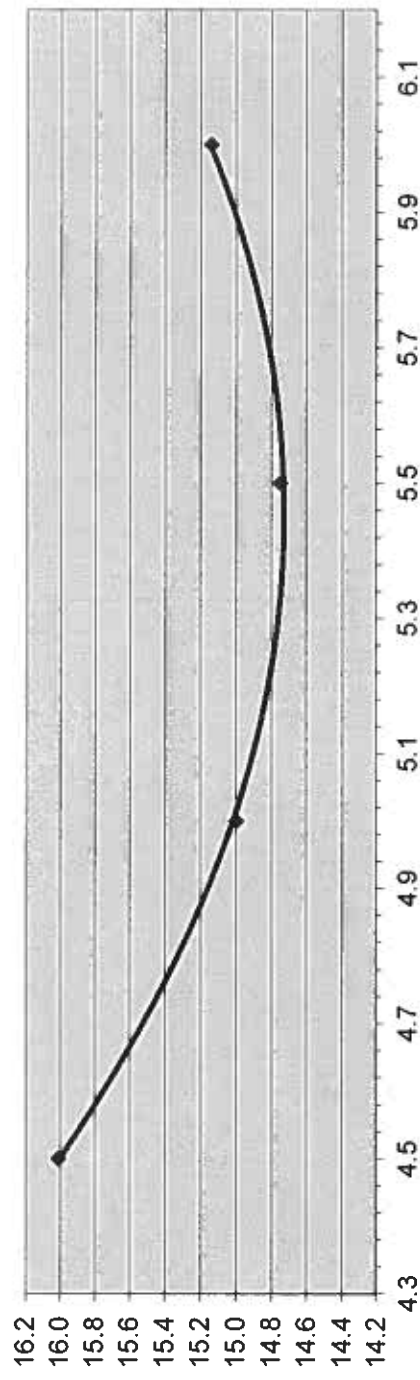
Asphalt Content (percent)	5.3	%	Lottman Specs:
Tensile Strength Retained (percent)	94	%	Min
Avg. Dry Tensile Strength (psi)	83.5	psi	80%
Avg. Cond. Tensile Strength (psi)	78.3	psi	30psi
Avg. Specimen Voids (percent)	6.8	%	Max
Avg. Saturation (percent)	96.5	%	6.0%
			8.0%



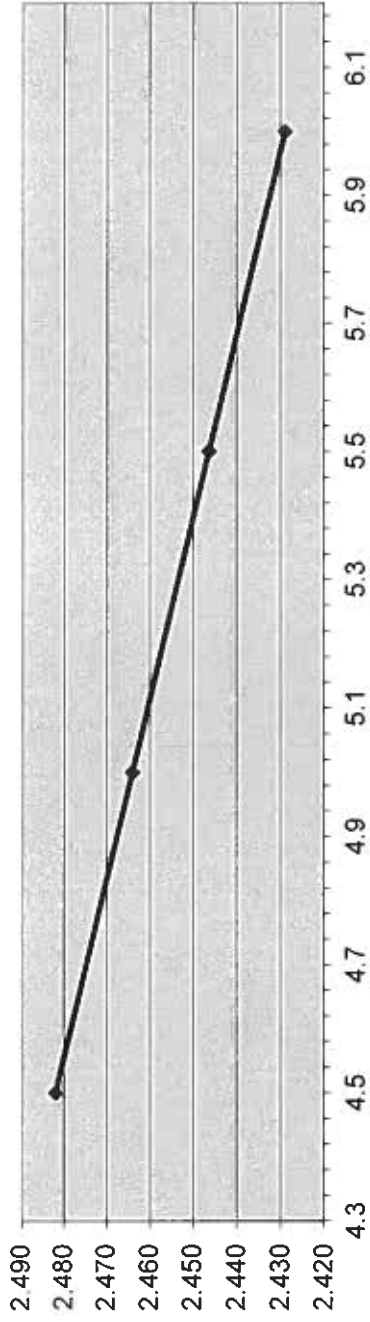
Voids vs. A.C. Content



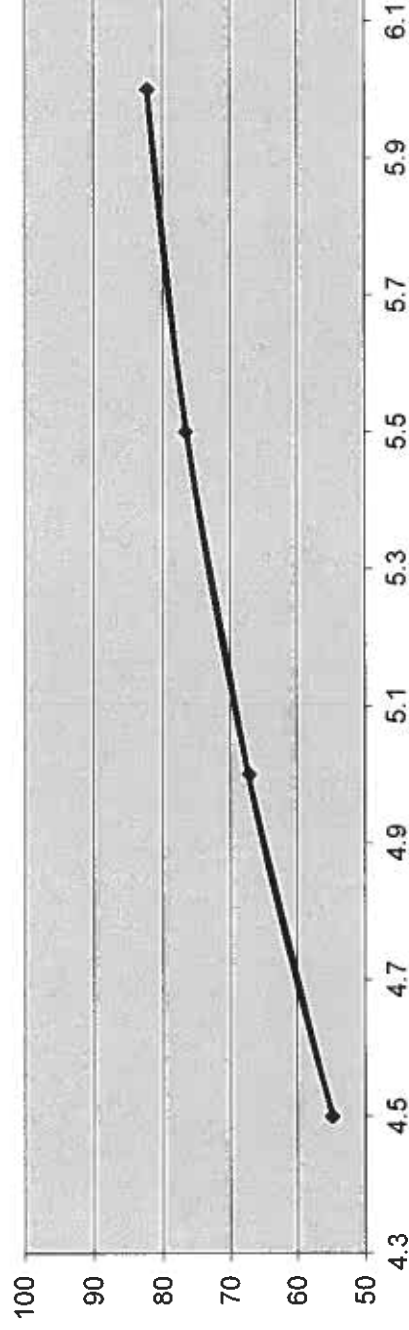
VMA vs. A.C. Content



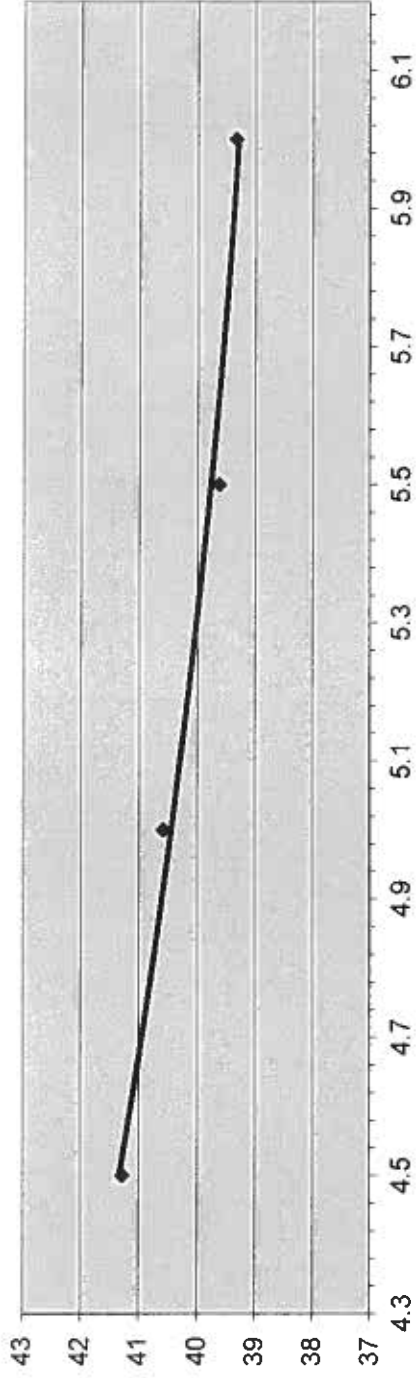
Max SpG vs. A.C. Content



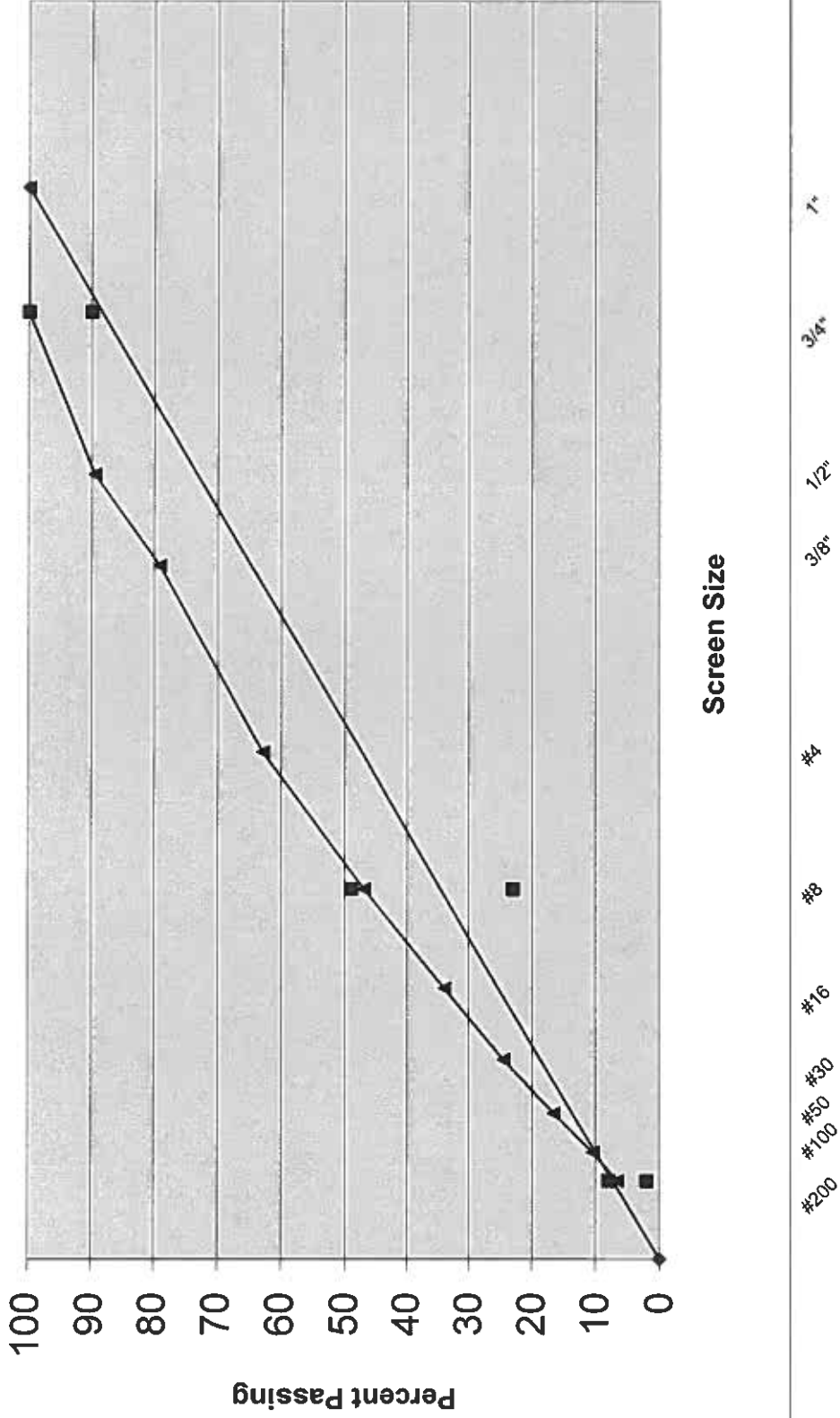
VFA vs. A.C. Content



Stability vs. A.C. Content



Aggregate Gradation



Sieve Size Raised to the .45 Power



2/15/13
 Schmidt Quality Control Laboratory
 Schmidt Construction Company
 Colorado Springs, Colorado 80910
 AMRL/AASHTO Accredited Laboratory No. 2531

Subject: Hot Mix Asphalt Mix Design
 SHRP Design Method – 75 Gyration
 Grading SX w/RAP PG64-22
 Delta Plant
 Mix Design No. SX75R6422-13

Attached are the results of a hot mix asphalt mix design performed in accordance with AASHTO, CDOT and/or ASTM procedures, utilizing the SHRP method for specimen compaction. Aggregates used in the mix design are from Menzer and Fountain Quarries. The asphalt binder is a PG 64-22, with a specific gravity of 1.038, a mixing temperature of 325F, and a compaction temperature of 300F, supplied by SUNCOR Materials. The anti-stripping additive used is 1% hydrated lime from Pete Lien.

The aggregate was blended to meet CDOT Grading SX criteria by blending 20% Fountain 1/2" HMA, 41% Menzer Asphalt Fines, 10% Menzer 1/2" Rock, 8% Menzer 3/4" Rock, 20% Crushed RAP and 1% Hydrated Lime. The individual gradations, combined blend and aggregate physical properties are presented on page one (1) and a graphical presentation of the volumetric properties and the combined gradation, plotted on a 0.45 Power Graph, begins on page four (4).

This design was performed using SHRP Mix Design guidelines. The design was performed at asphalt contents of 5.0, 5.5, 6.0 and 6.5 percent. The results of the tests performed at each asphalt content are presented on page two (2) of the design. Optimum values for our Standard Mix Design are chosen at 4.0% VTM and are presented on page two (2). Optimum values for the PPRAS Criteria Mix Design are chosen at 3.5% VTM and are presented on page three (3). Based on these results the properties of this mix are:

	VALUES @	STANDARD OPTIMUM	PPRAS OPTIMUM
Optimum AC Content, %		5.8	6.0
Theoretical Maximum Specific Gravity		2.435	2.426
Air Voids, %		4.0	3.4
Voids in Mineral Aggregate, %		15.9	15.9
Voids Filled with Asphalt, %		75	78
Tensile Strength Ratio		93	93

This hot mix asphalt mix design is based on specific materials and laboratory preparation of the test specimens. Variation between laboratory-produced and field-produced samples should be anticipated. It is recommended that the mix design be field verified during initial production. Field verification often results in the optimum asphalt cement content being adjusted to meet design air voids or voids in the mineral aggregate criteria.

If you have any questions concerning this mix design, please contact us at (719) 392-4207.

Performed By: Schmidt Quality Control Laboratory


 _____ Rueben Roberts, QC Manager

Reviewed By: 
 _____ Tom Blair, E.I.T.

Approved By: 
 _____ Rudy Keng, P.E.

3/12/2013



SCHMIDT CONSTRUCTION, COLORADO SPRINGS, CO
 Laboratory Design for Hot Bituminous Pavement-SHRP Method

AMRL/AASHTO Lab No. 2531

Mix Identification:

SX75R6422-13

Date:

2/15/13

Project:

2013 MIX DESIGN SX75R

Location:

DELTA PLANT

Contractor/Supplier: Schmidt Construction

Pit Name: Menzer Quarry / Fountain Pit

Grading: SX w/RAP 75 Gyration

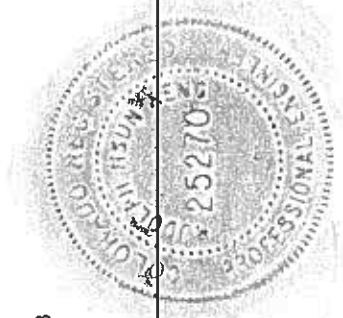
AC Source & Grade: SUNCOR PG 64-22 Mixing/Compacting Temperatures: 325F / 300F

Aggregate Data (T-11, T-27, T-30, T-37):

Aggregate Sampled by (T-2, T-48):

Aggregate Source	Fountain 1/2" HMA Rock	Menzer Asphalt Fines	Menzer 1/2" Rock	Menzer 3/4" Rock	Lime	Virgin Aggregate Gradation	RAP	Combined Gradation	Control Points
Percent in Mix	20	41	10	8	1	80	20	100	
Passing 1 1/2	100	100	100	100	100	100	100	100	100
Passing 1	(37.5)	100	100	100	100	100	100	100	100
Passing 3/4	(25.0)	100	100	100	100	100	100	100	100
Passing 1/2	(19.0)	100	100	100	100	100	100	100	100
Passing 3/8	(12.5)	100	100	53	100	95	99	96	90
Passing 4	(9.5)	100	86	16	100	89	93	89	100
Passing 8	(4.75)	75	86	2	100	65	73	66	100
Passing 16	(2.36)	56	61	1	100	47	60	49	28
Passing 30	(1.18)	39	43	1	100	33	45	36	58
Passing 50	(0.60)	26	31	1	100	24	33	26	100
Passing 100	(0.30)	15	21	1	100	16	23	17	100
Passing 200	(0.15)	8	13	1	100	10	14	11	100
	(0.075)	5.5	7.6	0.7	98.6	6.7	8.0	7.0	2.0

Plastic or Non-Plastic (T-90)	N/P	N/P	N/P	N/P	RAP AC %	4.45	Specs:
Aggregate Bulk SpG (T-84 & T-85)	2.582	2.621	2.619	2.611	2.380	2.619	Min. Max.
Aggregate App. SpG (T-84 & T-85)	2.650	2.680	2.687	2.661	2.380	2.668	60% 45%
Agg Water Abs (%) (T-84 & T-85)	0.98	0.85	0.96	0.72		0.86	45 45%
Aggregate Eff. SpG (T-84 & T-85)						2.661	18% 12%
Fine Agg. Bulk SpG (T-84)						2.608	
Coarse Agg. Bulk SpG (T-85)						2.615	
Blinder SpG						1.038	
Fractured Faces (CP-45, ASTM D-5821)						90	
Sand Equivalent (T-176)						62	
LA Abrasion (T-96)	37.6	33.6	32.8	30.9		33	
Fine Aggregate Angularity (T-304)						48	
Micro-Deval (CP-L 4211)	10.5	16.2	14.4	13.8		14	
Sodium Soundness (T-104)						1	
Flat & Elongated - Ratio 5:1 (ASTM D-4791)						0.0	
Flat & Elongated - Ratio 3:1 (ASTM D-4791)						0.2	



Standard

SCHMIDT CONSTRUCTION, COLORADO SPRINGS, CO
 Laboratory Design for Hot Bituminous Pavement-SHRP Method
 Standard Mix Design Criteria

Lab No. 2531
 Version

Mix Design A.C. Content Determination Results:

Rice =	2.428	@	6.0	%AC
A.C. Content	5.0	5.5	6.0	6.5
Rice Data (T-209)	2.463	2.445	2.428	2.411
Specimen SpG Data (CP-L5115 & CP-L5106):				
Bulk SpG @ Ninit	2.109	2.152	2.161	2.186
Bulk SpG @ Ndes	2.290	2.327	2.345	2.353
Height @ Ndes	64.5	64.0	63.3	62.9

Voids Specs:

Min	Max
3.5%	4.5%

Voids Data:

Voids @ Ninit	14.4	12.0	11.0	9.3	%
Voids @ Ndes	7.0	4.8	3.4	2.4	%

Other Data:

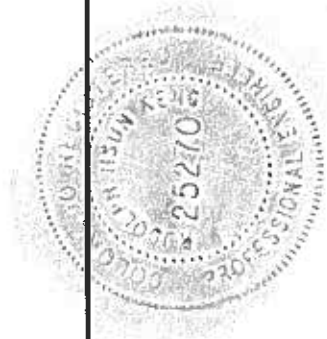
VMA @ Ndes (CP-48)	16.9	16.0	15.9	16.0	%
VFA @ Ndes	59	70	78	85	%
Aggregate Effective SpG (T-84 & T-85)	2.655	2.655	2.655	2.655	
Effective Asphalt Content	4.50	5.00	5.50	6.01	
Dust to A.C. Ratio (CP-50)	1.3	1.2	1.1	1.0	
Stability (CP-L5106)	39	39	38	37	

Optimum A.C. Content Results:

Optimum A.C. Content	5.8	%	Voids at Ninit at Optimum A.C.	11.4	%
Rice at Optimum A.C.	2.435		Voids at Ndes at Optimum A.C.	4.0	%
Hveem Stability	38		VMA at Optimum A.C.	15.9	%
Voids Filled with Asphalt (VFA)	75	%	Dust to Asphalt Ratio	1.1	

Lottman Moisture Sensitivity Results (CP-L5109, Method B)

Lottman Specs:	
Min	Max
Asphalt Content	80%
Tensile Strength Ratio	30psi
Avg. Dry Tensile Strength	110.6 psi
Avg. Cond. Tensile Strength	102.9 psi
Avg. Specimen Voids	6.8 %
Avg. Saturation	91.7 %
	6.0% 8.0%



SCHMIDT CONSTRUCTION, COLORADO SPRINGS, CO
 Laboratory Design for Hot Bituminous Pavement-SHRP Method
 Pikes Peak Regional Asphalt Specification Mix Design Criteria

Lab No.
Version

2531

Mix Design A.C. Content Determination Results:

Rice =	2.428	@	6.0	%AC
A.C. Content	5.0	5.5	6.0	6.5 %
Rice Data (T-209)	2.463	2.445	2.428	2.411

Specimen SpG Data (CP-L5115 & CP-L5106):

Bulk SpG @ Ninit	2.109	2.152	2.161	2.186
Bulk SpG @ Ndes	2.290	2.327	2.345	2.353
Height @ Ndes	64.5	64.0	63.3	62.9 mm

Voids Data:

Voids @ Ninit	14.4	12.0	11.0	9.3 %
Voids @ Ndes	7.0	4.8	3.4	2.4 %

Other Data:

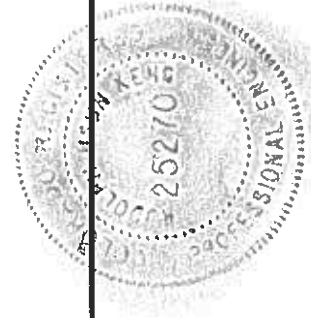
VMA @ Ndes (CP-48)	16.9	16.0	15.9	16.0 %
VFA @ Ndes	59	70	78	85 %
Aggregate Effective SpG (T-84 & T-85)	2.655	2.655	2.655	2.655
Effective Asphalt Content	4.50	5.00	5.50	6.01
Dust to A.C. Ratio (CP-50)	1.3	1.2	1.1	1.0
Stability (CP-L5106)	39	39	38	37

Optimum A.C. Content Results:

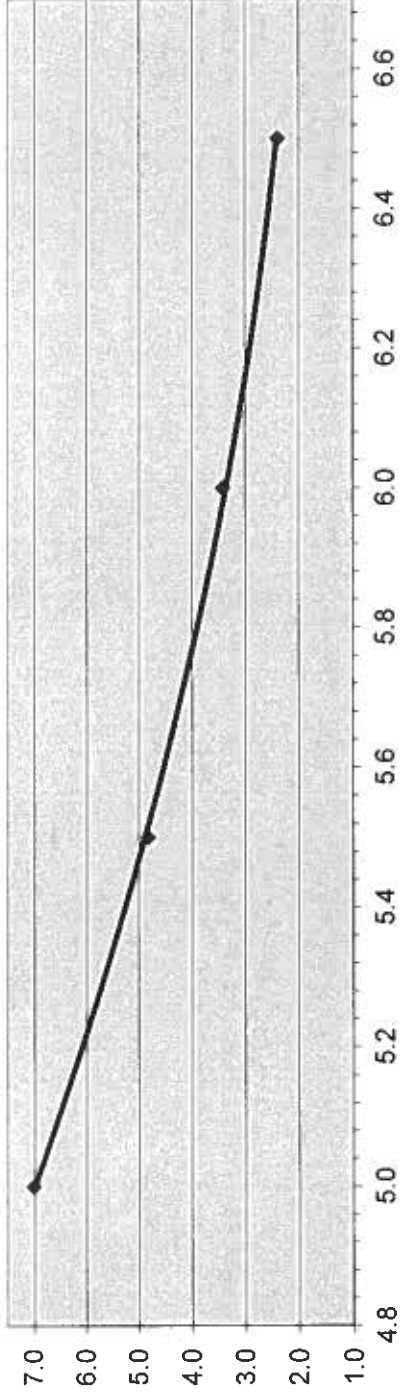
Optimum A.C. Content	6.0	%	Voids at Ninit at Optimum A.C.	11.0	%
Rice at Optimum A.C.	2.428		Voids at Ndes at Optimum A.C.	3.4	%
Hveem Stability	38		VMA at Optimum A.C.	15.9	%
Voids Filled with Asphalt (VFA)	78	%	Dust to Asphalt Ratio	1.1	

Lottman Moisture Sensitivity Results (CP-L5109, Method B)

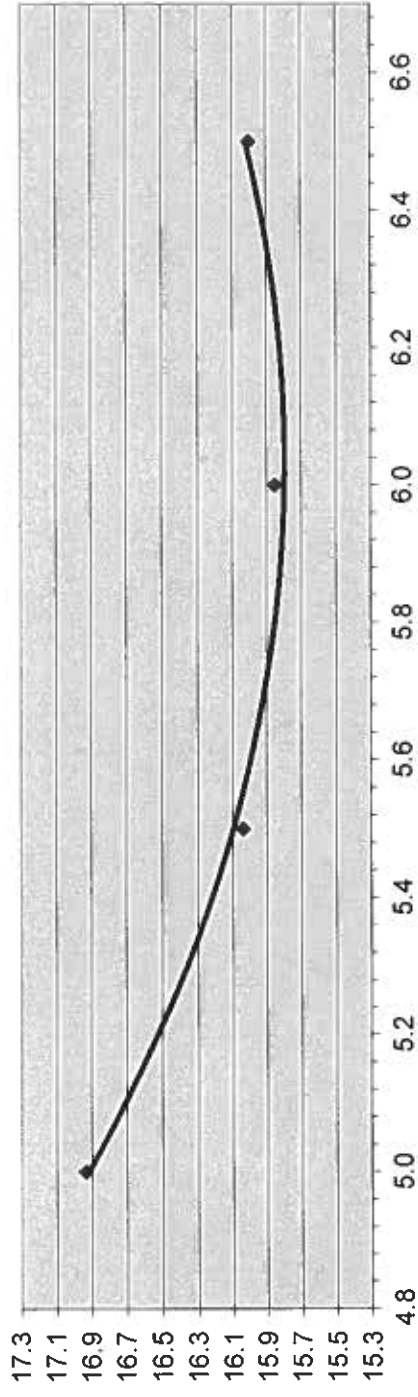
Asphalt Content	5.8	%	Lottman Specs:
Tensile Strength Ratio	93		Min
Avg. Dry Tensile Strength	110.6	psi	80%
Avg. Cond. Tensile Strength	102.9	psi	30psi
Avg. Specimen Voids	6.8	%	6.0%
Avg. Saturation	91.7	%	8.0%



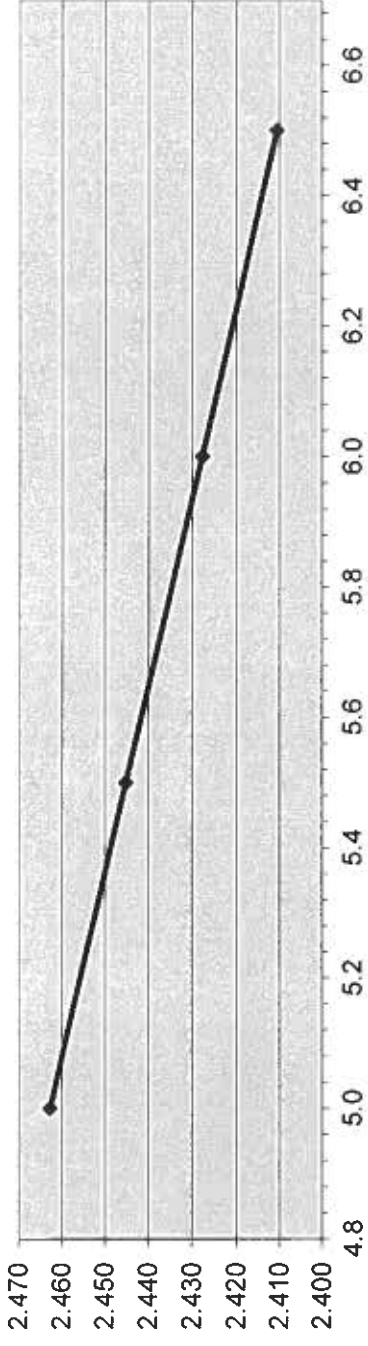
Voids vs. A.C. Content



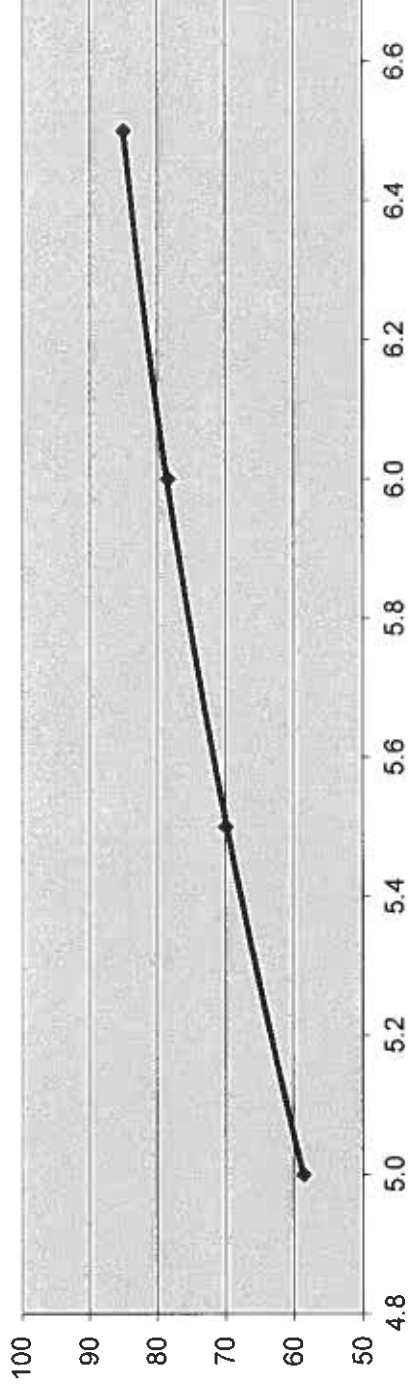
VMA vs. A.C. Content



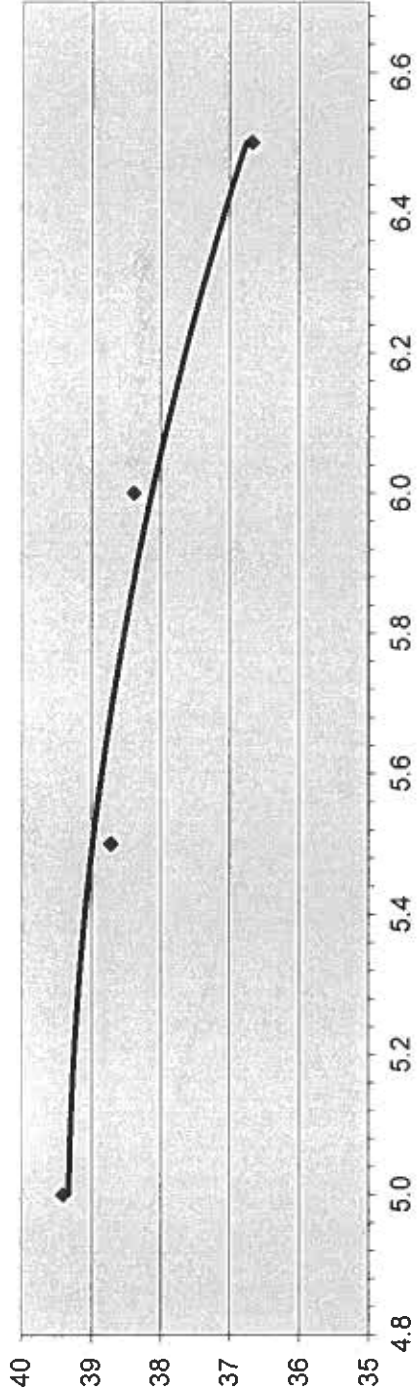
Max SpG vs. A.C. Content



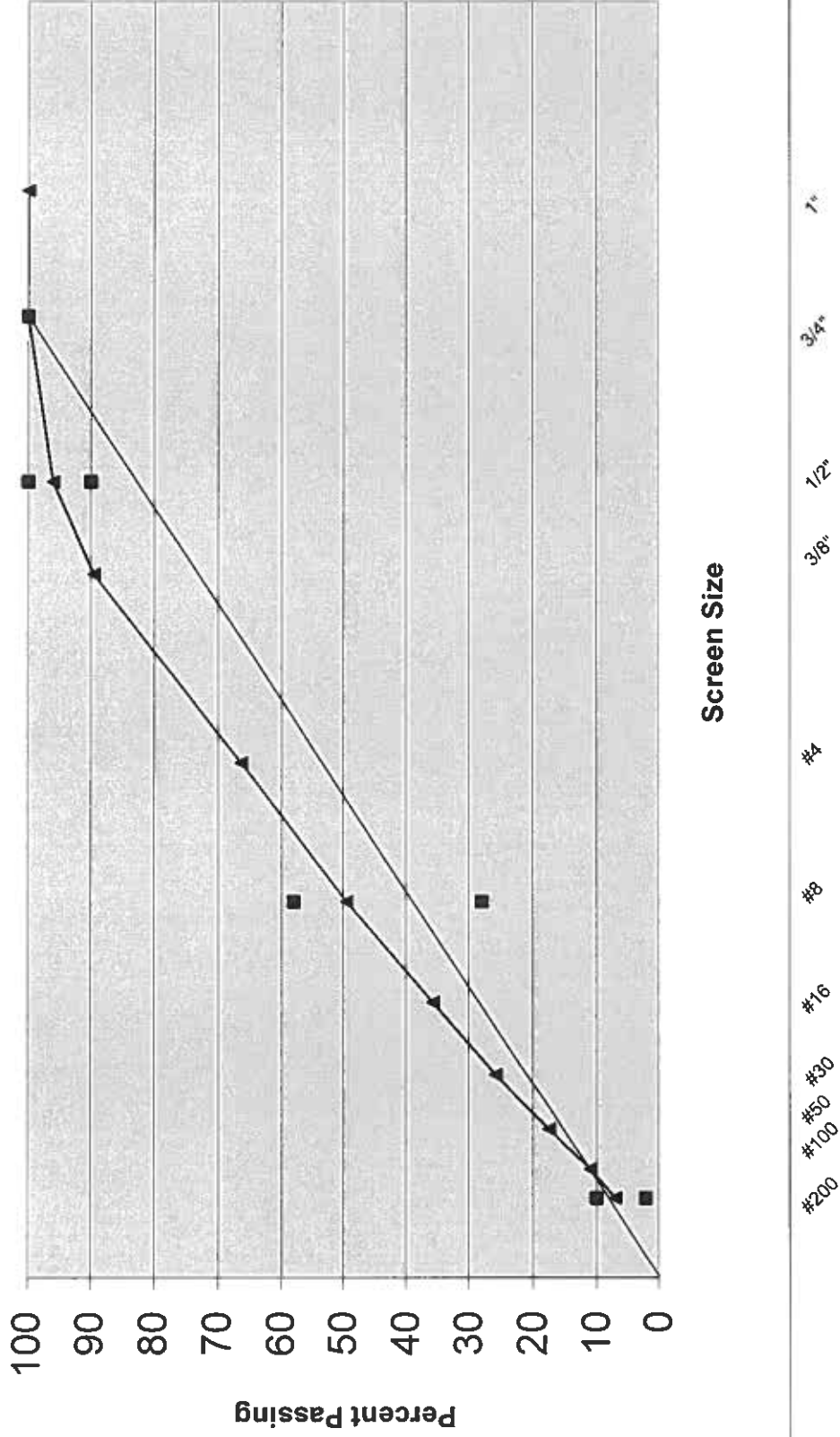
VFA vs. A.C. Content



Stability vs. A.C. Content



Aggregate Gradation



Sieve Size Raised to the .45 Power